CREW ENGINEERING AND SURVEYING

5725 KEARNY VILLA ROAD, SUITE D SAN DIEGO, CALIFORNIA 92123 (858) 571-0555

Preliminary Drainage Study Rua Mi chelle T.P.M.

Introduction

The proposed project, located at 25569 Rua Michelle in the Escondido area, is a subdivision of 5.68 gross acres in to four residential parcels of 1.29, 1.21, 1.07 and 2.11 acres each. The proposed parcels will be served by a private road. This is a lot sales project.

The existing topography of the site is moderately sloped. The westerly two-thirds of the property sheet flows to an existing drainage course that flows from north to south across the property. The easterly one third of the property drains via sheet flow to the east and south from an existing knob feature situated within the easterly one-third.

Grading has taken place on the property per County grading permit L-14349, that involved creating two non-structural pad areas, and the disposal of excess fill onsite.

This study is to estimate the developed runoff from the 100 year design event from and across the site and the surface drainage features that have been proposed to safely convey runoff and return it to sheet flow. The runoff exiting the site should remain the same following the development of this property.



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Thomas H. Koemer 9/2011

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Methodology

The limits of the drainage basins were determined using County topographic maps at 1"=200' scale. A site inspection and survey was conducted to verify the drainage basin, flow patterns and existing downstream drainage facilities.

The Rational Method (Q=CIA) was used to calculate the runoff.

Intensity based on 100 year frequency storm.

Equation for determining the time of concentration (Tc): Tc = Ti + Tt; Ti from Table 3-2 and $Tt = [11.9xL^3/H]^{0.385}$

Rational Method intensity calculation: 100 Year Frequency: P6 = 3.5" P24 = 8.0" P6 / P24 = 44%; Adjusted $P6 = 45\% \times P24 = 3.6$ ". P6 / P24 = 44%; $P6 = 7.44 \times P6 \times D^{-0.645}$

Pre-Development Drainage Basin Parameters

Basin	Length(mi)	Height (ft)	Ti (min)	Tt (min)	Tc (min)	С	l (in/hr)	Area (ac)	Q100(cfs)
Α	0.11	72	6.9	2.3	9.2	0.32	6.40	4.1	8.4
В	0.03	19	6.9	0.9	7.8	0.32	7.12	0.7	1.6
С	0.02	14	6.9	0.6	7.5	0.32	7.30	0.5	1.2
D	0.05	21	8.0	1.5	9.5	0.54	6.26	0.4	1.4

Post-Development Drainage Basin Parameters

Basin	Length(mi)	Height (ft)	Ti (min)	Tt (min)	Tc (min)	С	l (in/hr)	Area (ac)	Q100(cfs)
Α	0.11	83	11.5	2.2	13.7	0.40	4.95	4.1	8.1
В	0.04	26	11.5	1.1	12.6	0.38	5.23	0.7	1.4
С	0.02	9	6.9	0.5	7.4	0.32	7.36	0.5	1.2
D	0.05	21	8.0	1.5	9.5	0.55	6.26	0.4	1.4

Weighted Runoff Coefficient Calculation for Basin A, B & D:

$$C = 0.90x(\% Impervious) + C_px(1-\% Impervious)$$

Post-Development Basin A:

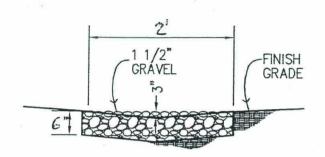
Post-Development Basin B:

Post-Development Basin D:

Conclusion

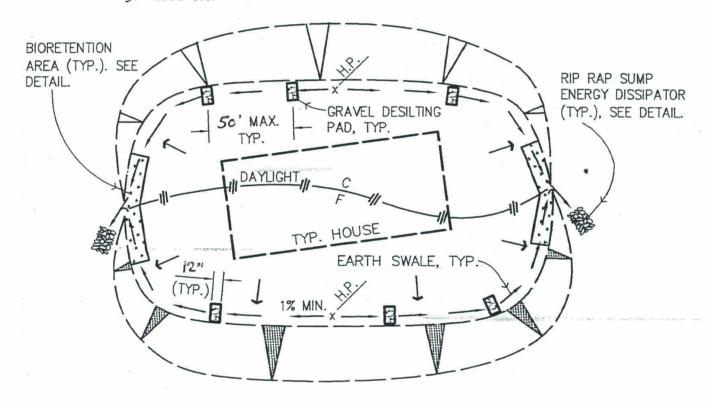
There will be no increase in runoff from the site due to construction of the project. Construction of the project will not result in any substantial erosion or detriment to downstream properties or drainage facilities. Energy dissipation devices are proposed, including rock filters around house pads, and rip rap sumps at the exit point of the pads and where runoff exits the driveways (see attached details). The drainage pattern of the site will not significantly change due to the construction of this project, and no drainage diversions will result from construction of the project as proposed.

Energy dissipation devices should be proposed, including rock filters around house pads, and grass lined swales and rip rap sumps at the exit point of the pads and where runoff exits the driveways (see attached details).



NOTE: INSTALL GRAVEL DESILTING PADS IN EARTH SWALE AT 50' MAX. O.C.

DETAIL OF GRAVEL DESILTING PAD

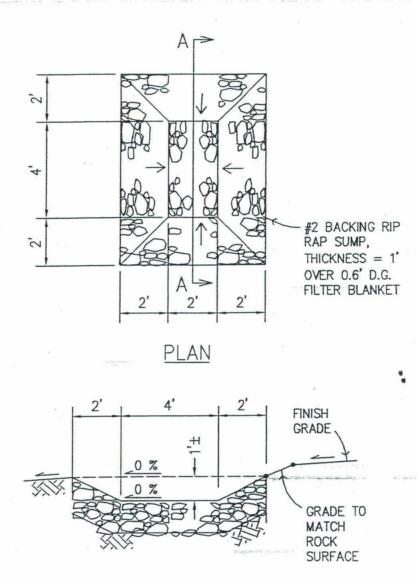


NOTE:

NO ROOF DRAIN PIPING OFF PAD. ALLOW WATER TO OVERLAND FLOW FROM DOWNSPOUT/SPLASHBLOCK THROUGH YARD.

TYPICAL PAD DRAINAGE DETAIL

NO SCALE



SECTION A-A

RIP RAP SUMP AT PAD OUTLET

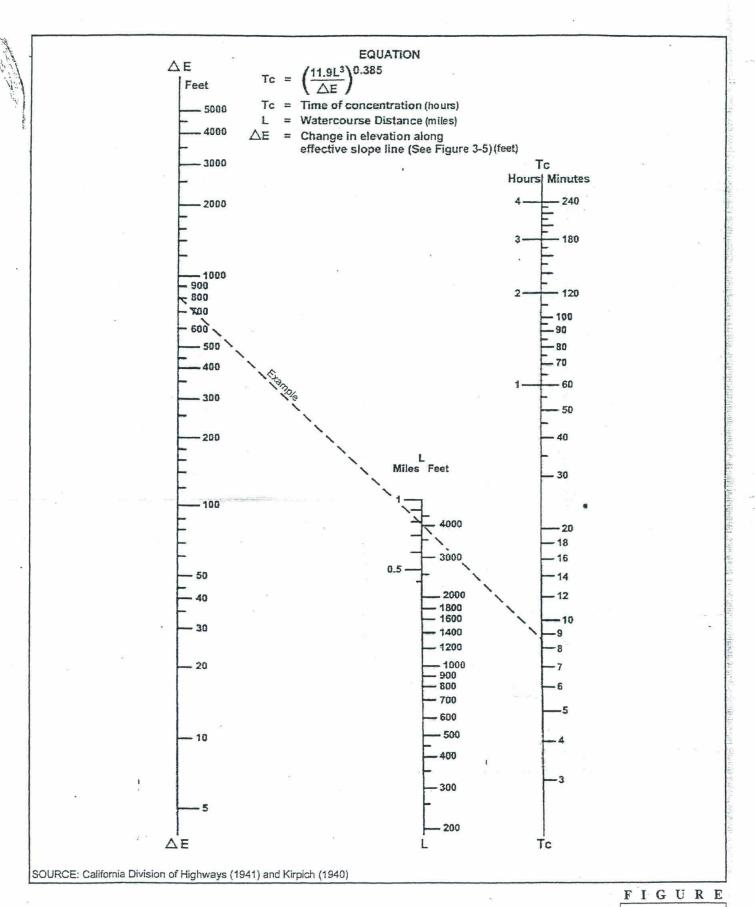
NOT TO SCALE

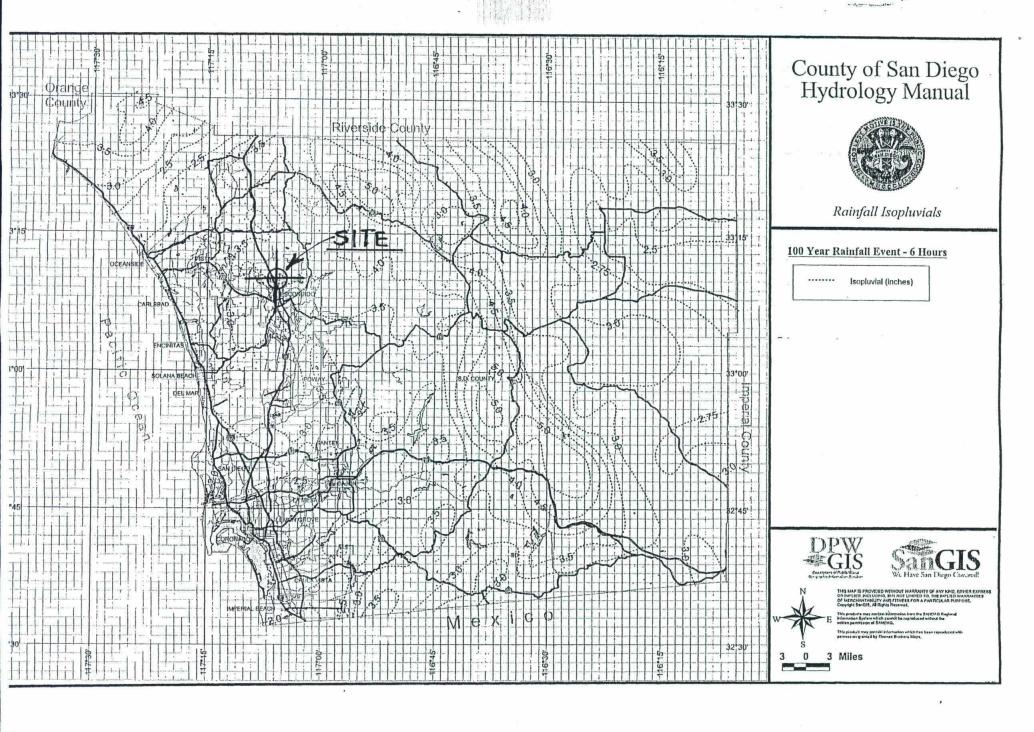
Table 3-2

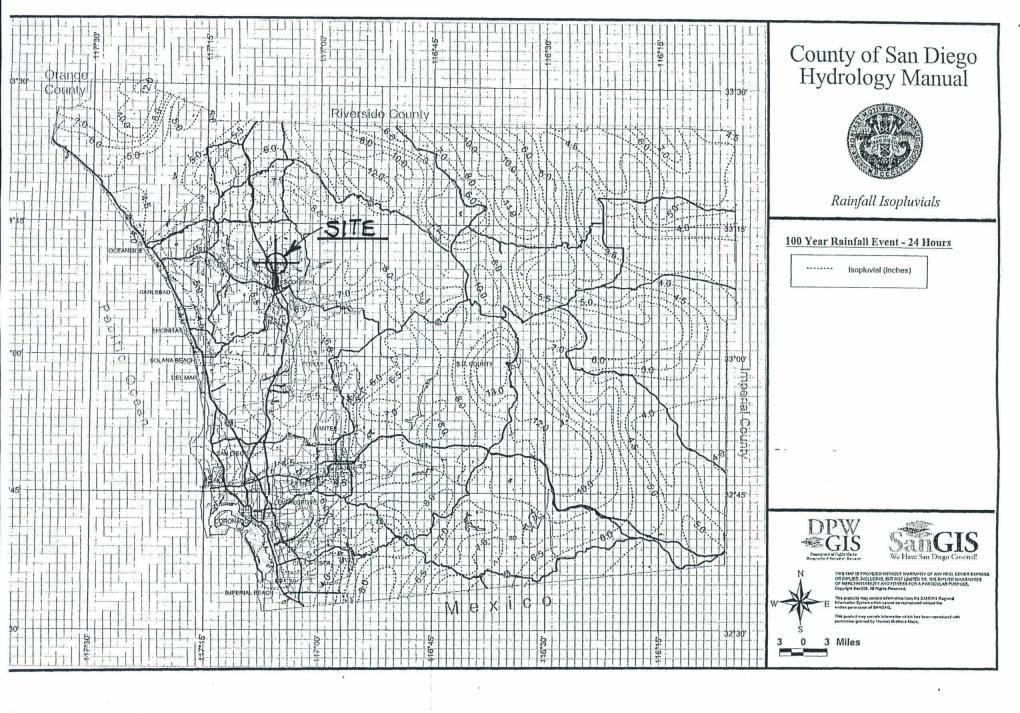
MAXIMUM OVERLAND FLOW LENGTH (L_M) & INITIAL TIME OF CONCENTRATION (T_i)

Element*	DU/		5%		%		2%		%		%	10)%
	Acre	L _M	Ti	L _M	Ti	L _M	Ti	L _M	Ti	L _M	Ti	L _M	Ti
Natural		50	13.2	70	12.5	85	10.9	100	10.3	100	8.7	(100	6.9)
LDR	1	50	12.2	(70	1.1.5	85	10.0	100	9.5	100	8.0	100	6.4
LDR	2	50	11.3	70	10.5	85	9.2	100	8.8	100	7.4	100	5.8
LDR	2.9	50	10.7	70	10.0	85	8.8	95	8.1	100	7.0	100	5.6
MDR	4.3	50	10.2	70	9.6	80	8.1	95	7.8	100	6.7	100	5.3
MDR	7.3	50	9.2	65	8.4	80	7.4	95	7.0	100	6.0	100	4.8
MDR	10.9	50	8.7	65	7.9	80	6.9	_90	6.4	100	5.7	100	4.5
MDR	14.5	50	8.2	65	7.4	80	6.5	90	6.0	100	5.4	100	4.3
HDR	24	50	6.7	65	6.1	75	5.1	90	4.9	95	4.3	100	3.5
HDR	43	50	5.3	65	4.7	75	4.0	85	3.8	95	3.4	100	2.7
N. Com		50	5.3	60	4.5	75	4.0	85	3.8	95	3.4	100	2.7
G. Com	2	50	4.7	60	4.1	75	3.6	85	3.4	90	2.9	100	2.4
O.P./Com		50	4.2	60	3.7	70	3.1	80	2.9	90	2.6	100	2.2
Limited I.		50	4.2	60	3.7	70	3.1	80	2.9	90	2.6	100	2.2
General I.		50	3.7	60	3.2	70	2.7	80	2.6	90	2.3	100	1.9

^{*}See Table 3-1 for more detailed description



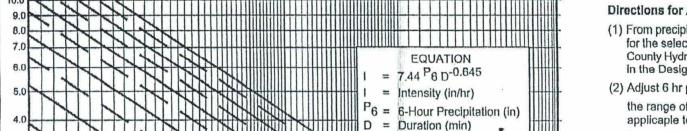












Directions for Application:

- (1) From precipitation maps determine 6 hr and 24 hr amounts for the selected frequency. These maps are included in the County Hydrology Manual (10, 50, and 100 yr maps included In the Design and Procedure Manual).
- (2) Adjust 6 hr precipitation (if necessary) so that it is within the range of 45% to 65% of the 24 hr precipitation (not applicable to Desert).
- (3) Plot 6 hr precipitation on the right side of the chart.
- (4) Draw a line through the point parallel to the plotted lines.
- (5) This line is the intensity-duration curve for the location being analyzed.

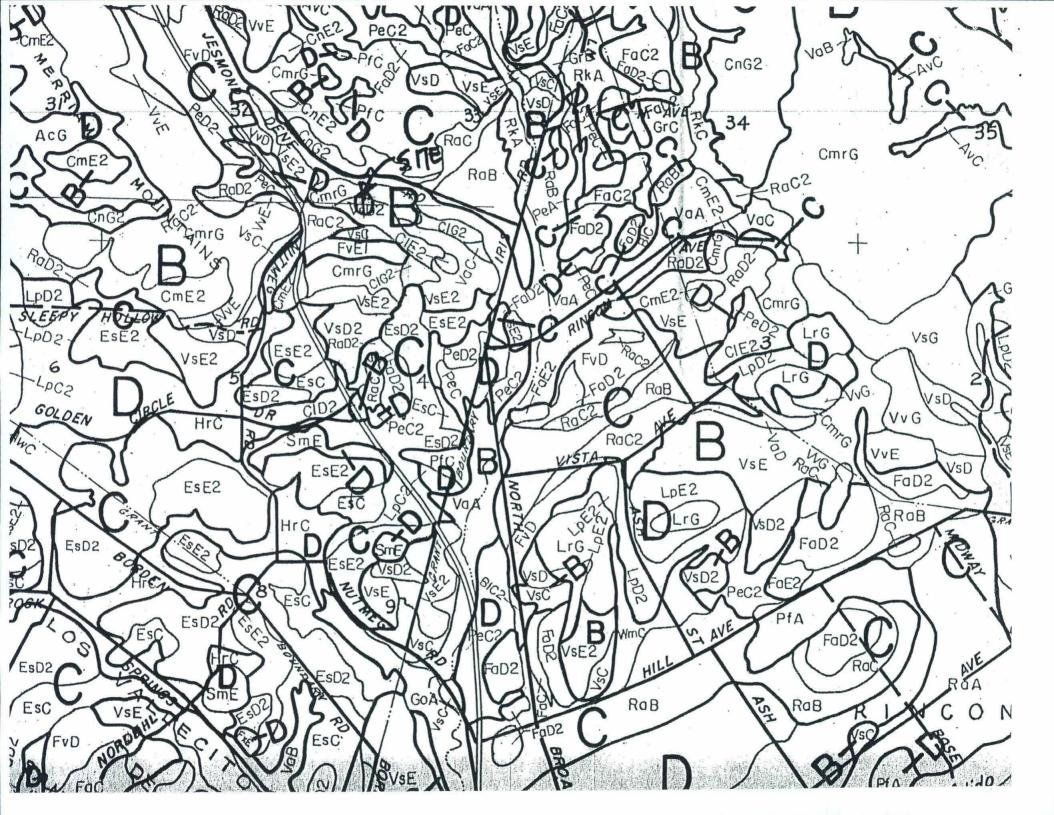
Application Form:

(a) Selected frequency
$$160$$
 year
(b) $P_6 = 3.5$ in., $P_{24} = 8.0$, $P_6 = 44$ %⁽²⁾
(c) Adjusted $P_6^{(2)} = 3.6$ in.
(d) $t_x = \frac{SEE\ CHART}{min.}$
(e) $I = \frac{SEE\ CHART}{in./hr}$

Note: This chart replaces the Intensity-Duration-Frequency curves used since 1965.

P6	1	1.5	2	2.5	3 -	3.5	4	-4.5	5	5.5	6
Duration	1	1	1	i	1	1	1	1	1	1	i
5	2.63	3.95	5.27	6.59	7.90	9.22	10.54	11.86	13.17	14.49	15.81
7	2.12	3.18	4.24	5.30	6.36	7.42	8.48	9.54	10.60	11.66	12.72
10	1.68	2.53	3.37	4.21	5.05	5.90	6.74	7.58	8.42	9.27	10.11
15	1.30	1.95	2.59	3.24	3.89	4.54	5.19	5.84	6.49	7.13	7.78
20	1.08	1.62	2.15	2.69	3.23	3.77	4.31	4.85	5.39	5.93	6.46
25	0.93	1.40	1.87	2,33	2.80	3.27	3.73	4.20	4.67	5.13	5.60
30	0.83	1.24	1.66	2.07	2.49	2.90	3.32	3.73	4.15	4.56	4.98
40	0,69	1.03	1.38	1,72	2.07	2.41	2.76	3.10	3.45	3.79	4.13
50	0.60	0.90	1.19	1.49	1.79	2.09	2.39	2.69	2.98	3.28	3.58
60	0.53	0.80	1.06	1.33	1.59	1,86	2.12	2.39	2.65	2.92	3.18
90	0.41	0.61	0.82	1.02	1.23	1.43	1.63	1.84	2.04	2.25	2:45
120	0.34	0.51	0.68	0.85	1.02	1.19	1.36	1.53	1.70	1.87	2,04
150	0.29	0.44	0.59	0.73	0.88	1.03	1.18	1.32	1.47	1.62	1.76
180	0.26	0.39	0.52	0.65	0.78	0.91	1.04	1.18	1.31	1.44	1.57
240	0.22	0.33	0.43	0.54	0.65	0.76	0.87	0.98	1.08	1.19	1.30
300	0.19	0.28	0.38	0.47	0.56	0.66	0.75	0.85	0.94	1.03	1.13
360	0.17	0.25	0.33	0.42	0.50	0.58	0.67	0.75	0,84	0.92	1.00

FIGURE



San Diego County Hydrology Manual Date: June 2003

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Table 3-1 RUNOFF COEFFICIENTS FOR URBAN AREAS

Laı	nd Use		Runoff Coefficient "C"								
			Soil Type								
NRCS Elements	County Elements	% IMPER.	A	В	- C	D					
Undisturbed Natural Terrain (Natural)	Permanent Open Space	0*	0.20	0.25	0.30	0.35					
Low Density Residential (LDR)	Residential, 1.0 DU/A or less	10	0.27	0.32	0.36	0.41					
Low Density Residential (LDR)	Residential, 2.0 DU/A or less	20	0.34	0.38	0.42	0.46					
Low Density Residential (LDR)	Residential, 2.9 DU/A or less	25	0.38	0.41	0.45	0.49					
Medium Density Residential (MDR)	Residential, 4.3 DU/A or less		0.41	0.45	0.48	0.52					
Medium Density Residential (MDR)	Residential, 7.3 DU/A or less	40	0.48	0.51	0.54	0.57					
Medium Density Residential (MDR)	Residential, 10.9 DU/A or less	45	0.52	0.54	0.57	0.60					
Medium Density Residential (MDR)	Residential, 14.5 DU/A or less	50	0.55	0.58	0.60	0.63					
High Density Residential (HDR)	Residential, 24.0 DU/A or less	65	0.66	0.67	0.69	0.71					
High Density Residential (HDR)	Residential, 43.0 DU/A or less	80	0.76	0.77	0.78	. 0.79					
Commercial/Industrial (N. Com)	Neighborhood Commercial	80	0.76	0.77	0.78	0.79					
Commercial/Industrial (G. Com)	General Commercial	85	0.80	0.80	0.81	0.82					
Commercial/Industrial (O.P. Com)	Office Professional/Commercial	90	0.83	0.84	0.84	0.85					
Commercial/Industrial (Limited I.)	Limited Industrial	90	0.83	0.84	0.84	0.85					
Commercial/Industrial (General I.)	General Industrial	95	0.87	0.87	0.87	0.87					

^{*}The values associated with 0% impervious may be used for direct calculation of the runoff coefficient as described in Section 3.1.2 (representing the pervious runoff coefficient, Cp, for the soil type), or for areas that will remain undisturbed in perpetuity. Justification must be given that the area will remain natural forever (e.g., the area is located in Cleveland National Forest).

DU/A = dwelling units per acre

NRCS = National Resources Conservation Service

